Evaluation of Aquifer Porosity And Hydraulic Conductivity From Empirical Equations Using Geoelectrical Sounding Measurements In Piramagroon Area NE-Iraq

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Abstract:

Application of Vertical Electrical Sounding (VES) with Schlumberger array as a low-cost technique and veritable method in groundwater exploration is more suitable for hydrogeological survey of sedimentary basins. This method is regularly used to solve a wide variety of ground water problems and hydraulic parameters. The main objective of this research therefore, is to evaluate aquifer porosity and hydraulic conductivity using the empirical equations of porosity and hydraulic conductivity with resistivity conducted in the continuation of the adjacent Sharazoor basin. For this purpose, four profiles were taken in studied area (Piramagroon district), and each profile includes five VES points of measurements. Then each VES was interpreted manually as well as by IPI2 win program for determining aquifer depth ranging from (40 m.) to (80 m.) in piramagroon district) and resistivity values range between (37.0 Ω .m) to (102 Ω .m), which are substituted in the empirical porosity-resistivity and hydraulic conductivity-resistivity values range along the studied area range between (21%) to (39 %), and for hydraulic conductivity values range from (1 m/day) to (4 m/day), which shows the increasing of the both aquifer porosity from the top of uplifted subsurface layers underlying the piramagroon district toward both limbs according to increasing of rock fragments (gravel, pebble) and (sand sediments) and decreasing of clay content overlying upper part of Middle Tanjero Formation.

Key Words: Aquifer Porosity (ϕ), Hydraulic Conductivity (K), (ϕ) & (K) Equations, Vertical Electrical Sounding.

1. Introduction:

Porosity is a measure of the amount of void space in a porous medium. It is defined as the ratio of the pore volume to the total volume of the sample and thus has no unit. When a medium contains isolated pores, the effective porosity (commonly considered to represent the pore space available for fluid flow) is less than the total porosity (De block, 2013).

The pore space is often considered in terms of individual pores-an artificial concept that enables quantifications of its essential character. Though many alternatives could serve as a basis for the definition of pores and their sizes, in soil science and hydrology these are best conceptualized, measured, and applied with respect to the fluids that occupy and move within the pore space, (Nimmo, 2004).

The development of groundwater resources and the regime of its activity largely depend on the porosity and permeability of water bearing formations. The porosity of rock is a measure of the amount of interstitial space that is capable of holding fluids and the permeability (hydraulic conductivity) of a rock is a quantitative measure of the case with which it will permit the passage of fluids through it under a hydraulic gradient. The determination of aquifer characteristics such as hydraulic conductivity and transmissivity is best made on the basis of data obtained from test pumping wells, (Batayneh, 2009).

Vukovic and Soro, (1992) noted that the applications of different empirical formulae to the same porous medium material can yield different values of hydraulic conductivity, which may differ by a factor of 10 or even 20. The objective of their research is therefore to evaluate the applicability and reliability of some of the commonly used empirical formulae for the determination of hydraulic conductivity of unconsolidated soil and rock materials.

(Odong, 2007) was shown that all the results of seven empirical formulae reliably estimated hydraulic conductivities of the various soil samples well within the known ranges.

Urumovic, K, and Urumovic, S. K, (2016) objective of their article is to research the relationship between average mean grain size and effective porosity in relation to permeability and specific surface area for a wide range of grain sizes and particle uniformities in various soil samples. In the hydraulic conductivity calculations, the Kozeny-Carman equation was used to discover the algorithm for calculating the referential mean grain size. This grain size, along with effective porosity, generates a harmonious parametric concept of the impact of porous media geometrics on its transmission capacity.

2. Study area and field technique

The applicable studied area known as Piramagroon district locates NE-Iraq and North West of Sulaimani city. The area was surveyed along four profiles (A, B, C and D) and conducting twenty VES-points distributed throughout the area by using symmetrical Schlumberger electrode arrangement, as shown in Fig.(1).

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Fig. (1) Location map of studied area with the surveying profile drawn

Measurements of VES-points were taken by modern computerized static resistivity meter (SYSCAL Jr-Switch-72) type as shown in fig.(2), with the half length of spread (AB/2 = 500 m.) and (MN/2 = 80 m.) in order to increase depth of penetration to required depth possible to detect layers depths and thicknesses as well as corresponding resistivities. The instrument is a new multimode combined system of switching unit (link box, current transducer, potentiometer with attached rechargeable battery). The instrument is controlled by a microprocessor for conducting one and two dimensional electrical resistivity survey by using of (Wenner, Schlumberger and Dipole-Dipole) or any other arrangement. It can be also used for conducting Spontaneous polarization (SP) method and induced polarization.



Fig. (2) Modern computerized static resistivity meter (SYSCAL Jr-Switch-72) drawn by Surfer 8, 2002

Theoretical formulas for aquifer porosity (φ) and hydraulic conductivity estimation Formulas for aquifer porosity (φ) estimation

The porosity of a rock or soil is a measure of the contained interstices or void expressed as the ratio of the volume of interstices to the total volume, (Todd and Mays, 2005), if (ϕ) is porosity, then:-

Where, $Vv_{\mathcal{J}} \underline{volume} \ \underline{bf} \ \overline{vold}, \ \underline{Vs}; \ \underline{volume} \ \underline{of} \ \underline{solid}; \ V_T; \ Total \ volume$

Archie, (1942) formulated an empirical equation, in which states that the resistivity of the aquifer (ρ_r) is directly proportional to the resistivity of the water (ρ_w) filling the pores of the sand stone Formation, the proportionality constant (F) is known as the formation factor as follow:-

$$\rho_r \alpha \rho_w$$

So that:-

$$\rho_r = F \rho_w$$

According to the Archie Approximation factor (F) relationship is given by:-

Equation (3) care be rearranged in term $qB(\phi)$, as follow:-

$$\phi^{-m} = \frac{F}{a}$$
$$\frac{1}{\phi^{m}} = \frac{F}{a}$$
$$\phi^{m} = a$$

 $\phi^{m} = \frac{a}{dt}$ DOI:https://doi.org/10.26750/Vot(7).No(4).paper16 <u>http://journal.uor.edu.krd/index.php/JUR Vol.7. No.4</u>, December.2020

By powering both sides of the Eq. to $(\frac{1}{m})$:-

$$\left(\phi^{m}\right)^{\frac{1}{m}} = \left(\frac{a}{F}\right)^{\frac{1}{m}}$$
$$\phi = \left(\frac{a}{F}\right)^{1/m} \cdots \cdots \cdots \cdots (4)$$

Vukovic and Soro,(1992) were put an empirical formula for estimating hydraulic conductivity from porosity presented in a general formula:-

Where:-

K = hydraulic conductivityg = acceleration due to gravityv = kinematics viscosityC = sorting coefficient;f(n) = porosity function, $d_e =$ effective grain diameter.The kinematics viscosity (v) is related to dynamic viscosity (μ) and the fluid (water) density (ρ) as follows:

If eq. (6) substitutes in eq. (5) it will become:-

$$K = \frac{\rho \cdot g}{\mu} \cdot c \cdot f(n) \cdot d_e^{-2} \dots \dots \dots (7)$$

Eq. (7) can be put in form of porosity function f(n) as follow:-

$$f(n) = \frac{K \cdot \mu}{\rho \cdot g} c.d_e^{-2}....(8)$$

Eq. (8) can be used for porosity estimation in case of knowing hydraulic conductivity (K) and both values of (C) and (d_e) are dependent on the different methods used in the grain-size analysis, and density of water is taken as ($\rho = 1000 \text{ kg/m}^3$), so this technique is also limited because it needs to know all the above mentioned parameters which will lead to restrict its application.

Typical range values of gravel with sand modified from (Kruseman and De Ridder, 1994; Bernard, 2003) as shown in Table (1).

Table (1) Typical values of porosity ranges within gravel, sand and sandstone

Materials and Rocks	Porosity (%)
Soils	50 - 60
Clay	45 - 55
Silt	40 - 50
Medium to coarse mixed sand	35 - 45
Uniform sand	30 - 40
Fine to medium mixed sand	30 - 35
Gravel	30-40
Gravel with sand	20 - 35
Sandstone	10 - 20
Shale	1 - 10

3.2 Formulas for aquifer hydraulic conductivity (K) estimation

Hydraulic conductivity of the sediments or rock is a measure of the ability of geologic unit (media) to transmit water, which is a function of both the medium and the fluid properties (Fetter, 1998). Whereas (Deming, 2002) had been returned hydraulic conductivity to a variety of physical factors, including porosity, particle size and distribution, shape of particles, arrangement of particles and other factors.

Transmissivity (T) can be calculated by conducting pumping test, through which Hydraulic conductivity (K) can be determined from calculated Transmissivity (T), by knowing the thickness of the saturated aquifer (H), (Daves and Dewiest, 1966), according to the following equations:-

T = K H (9)

Which can be expressed in term of (K), as follow:-

$$K = \frac{T}{H} \cdots \cdots \cdots \cdots (10)$$

Furthermore transmissivity (T) and hydraulic conductivity (K) also can be determined through the applying of (AQTESOLV) software program, which is supported the obtained manual results.

(Fatoba, et al., 2014) had demonstrated in their study the efficacy of surface geophysics in estimating hydraulic characteristics of an aquifer where pumping test data are not available and also to determine its vulnerability to surface contaminants.

The basic equations for geoelectrical exploration are developed assuming that the medium is porous, the matrix is generally an insulator and electrical currents flows through the water present in the pore spaces. The aquifer's electrical resistivity is mainly influenced by porosity and fluid resistivity in the pores. The geoelectrical data recorded on the surface therefore contain useful information about the aquifer which can be interpreted by experienced geophysicists for hydrogeological studies, (Sri Niwas and Muhammed, 2012).

Since the spatial distribution of aquifer properties cannot be confidently calculated in areas with few pumping tests, determination of aquifer parameters from geoelectric sounding becomes a good alternative and cost-efficient technique since drilling of wells to evaluate aquifer parameters can be both expensive and time-consuming. The range of aquifer parameters values obtained from VES interpretation is a good indication of the reliability of this method (Nicaise, et al., 2013).

The aquifer parameters values is supported by Urumovic, K, Urumovic Sr., K., (2016) through which they suggest procedures for calculating referential grain size and determining effective (flow) porosity, which result in parameters that reliably determine the specific surface area and permeability. These procedures ensure the successful application of the Kozeny – Carman model up to the limits of validity of Darcy's law. The value of effective porosity in the referential mean grain size function was calibrated within the range of 1.5 μ m to 6.0 mm. The reliability of the parameters applied in the KC model was confirmed by a very high correlation between the predicted and tested hydraulic conductivity (R²) values for sandy and gravelly materials is (0.99), whereas for clayey-silty materials is (0.70).

Sri Niwas and Lima, (2003) was concluded that electrical conductivity will be obey Archie's equation and the hydraulic conductivity can be estimated as for a single – phase conductor. Fortunately, under this condition, the aquifer would be termed saline, where electrical conductivity and hydraulic conductivity are in direct relationship, i.e. the porosity and electrical conductivity will obey a direct relationship.

Nelson, (2015) was concluded that groundwater is in constant motion, although the rate at which it moves is generally slower than it would move in a stream because it must pass through the intricate passageways between free spaces in the rock. First, the groundwater moves downward due to the pull of gravity. But it can also move upward because the water moves from high pressure zones to low pressure zones, and The velocity, V, is of groundwater flow is given by:-

where (K) is the *hydraulic conductivity*, which is a measure of the permeability of the material through which the water is following, If multiply this expression by the area (A) through which the water is moving, and then we get the discharge (Q).

Equation (12) represents Darcy Law, which simply states that discharge is proportional to the hydraulic gradient $(A^*(h_2-h_1)/L)$ times the hydraulic conductivity (K). Note that the stream discharge (Q) unit is $(m^3/sec.)$. The hydraulic conductivity variation due to the change of the grain size particles of unconsolidated sediments modified from (Bouwer, 1978 in Kruseman, and De Ridder, 1994), as shown in Table (2).

Unconsolidated materials and rocks	Hydraulic conductivity K (m / day)		
Clay	$10^{-8} - 10^{-2}$		
Fine Sand	1-5		
Medium sand	5-20		
Coarse Sand	20-100		
Gravel	100 - 1000		
Sand and gravel mixes	5-100		
Sandstone	$10^{-3} - 1$		
Carbonate rock with secondary porosity	$10^{-2} - 1$		
Shale	10 -7		
Dense solid rock	< 10 ⁻⁵		

Table (2) Range of the hydraulic conductivity of unconsolidated sediments

4. Result of interpretation field data

The field data of VES-points along profiles were interpreted manually using partial matching by Ebert method, and then the results were interpreted with software (IPI2 win) through the applying of forward calculation and inverse modeling programs for the purpose of measuring the reliability of the results.

The (IPI2 win) program is able to modify the input parameters of the measured apparent resistivities and AB/2 values. The output results of the true resistivities and thicknesses are reflected the best fit between field curve and one of the theoretical master curves, through the applying forward calculation program.

For more accuracy results of manual interpretation, an inverse modeling was also applied by entering of the manual results of the true resistivities and thicknesses. Then best fits are obtained, through applying of iteration

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process between the field curve and master curve with minimum percent of deviations. VES-points are shown in Figures (3), (4)



Fig.(3) Shows Inverse modeling output of IPI2 win -Software program and manual interpretation for VES – A1 of (HKHA) type

Then each VES was interpreted manually as well as by IPI2 win program, through which the results reveal that the aquifer depth ranging from (40 m.) in piramagroon district to more than (80m.). as well as realativity values range between (37 Ω .m) to (102 Ω .m), as shown in Table (3).



Fig.(4) Shows Inverse modeling output of IPI2 win-Software program and manual interpretation for VES – D5 of (HA) type

5. Result of porosity and hydraulic conductivity evaluation by empirical equations5.1 porosity estimation from empirical porosity-resistivity equation

(Amin, 2008) was deduced the positive linear relation between aquifer porosity and the measured resistivity from (8) pumping test wells assurance relation coefficient (R^2 =0.9827) in the continued Zharazoor basin of the studied area the formulated relation is given by the following equation.

$\phi = 0.2637\rho + 11.312\cdots(13)$

The direct porosity estimations from aquifer resistivity values calculated on the bases of the interpreted field data both manually and by applying computer program are measured reaiativity values within the range of (3 Ω .m) to (102 Ω .m), then by substituting of aquifer resistivity values in equation (13) the deduced aquifer porosity distributed along the studied area range between (21 %) to (38 %) as portrayed in Table (3). Abu Heen and Muhsen, 2017) had estimated Porosity to be in the range of (38 % – 52 %) with an average value of (46 %) which was close to the range of the present study.

5.2 Hydraulic conductivity evaluation from empirical equation

Based on estimating pumping tests for (18) wells. Amin, 2008 was deduced the positive hyperbolic relationship with relation coefficient ($R^2 = 0.72$) for hydraulic conductivity evaluation from measured resistivity of the aquifer from the corresponding following equation:-

 $K = 10^{0.0086 \ \rho - 0.32} \ \cdots \ (14)$

Also by substituting of the calculated aquifer resistivity values in equation (14) the deduced and hydraulic conductivity values will range from (1 m/day) to (4 m/day), shows the increasing of the both aquifer porosity hydraulic conductivity values from the top of uplifted subsurface layers underlying the piramagroon district toward both limbs according to increasing of rock fragments (gravel, pebble) and materials (sand) and decreasing of clay content overlying upper part of Middle Tanjero Formation.

Profiles	VES No.	Aquifer depth Z (m.)	Aquifer Resistivity $\rho_r (\Omega.m)$	Porosity φ=0.2637*ρ+11.312 φ (%)	Hydraulic conductivity $K=10 \land 0.0086*\rho - 0.32$ K (m / day)
Profile - A	A1	83.0	62.8	27.8	1.6
	A2	71.8	55.7	26.0	1.4
	A3	58.0	50.6	24.6	1.30
	A4	44.7	42.9	22.6	1.12
	A5	40.5	37.5	21.2	1.00
Profile - B	B1	95.7	72.8	30.5	2.02
	B2	86.8	66.7	28.9	1.80
	B3	75.0	58.4	26.7	1.52
	B4	60.4	51.6	24.9	1.33
	B5	45.7	40.8	22.0	1.07

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Table		C1	48.0	55.5	25.9	1.44	(3)
		C2	57.8	62.7	27.8	1.65	
	Profile - C	C3	69.9	65.7	28.6	1.75	
		C4	84,6	70.6	29.9	1.94	
		C5	98.6	74	30.8	2.07	
		D1	55.0	53.7	25.4	1.38	
		D2	69.3	65.6	28.6	1.75	
	Profile - D	D3	82.4	74	30.8	2.07	
		D4	95.4	87.5	34.3	2.70	
		D5	112.0	102	38.2	3.60	

Calculated aquifer porosity and hydraulic conductivity of the studied area

Increasing of the both aquifer porosity and hydraulic conductivity values from the top of uplifted subsurface layers underlying the piramagroon district toward both limbs clearly indicate of increasing of rock fragments (gravel, pebble) and materials (sand) and decreasing of clay content

overlying upper part of Middle Tanjero Formation. As a result of this the area of the piramagroon can be considered as the recharge zone for infiltrating surface rain water toward Tabin and piramagroon to subsurface saturated unconfined aquifer as shown in the following Fig. (5).



Fig. (5) Geological section shows aquifer situation under the studied area (Piramagroon district) drawn by Surfer 8, 2002

The estimated aquifer porosity values range along the studied area are range between (21%) to (39%) and from the porosity map the situation clearly indicates an increasing of porosity toward both limbs of the uplifted layers under Piramagroon district, as shown in Fig. (6).



Fig. (6) Aquifer porosity map of the studied area drawn by Surfer 8, 2002

The estimated hydraulic conductivity values range from (1 m/day) to (4 m/day) along the studied area from the hydraulic conductivity map the situation will also clearly indicates an increasing of hydraulic conductivity to ward both limbs of the uplifted layers under Piramagroon district, as shown in Fig. (7).



Fig. (7) Hydraulic conductivity map of the studied area drawn by Surfer 8, 2002

6. Conclusion:

Since drilling of wells to determine hydraulic parameters is often prohibitively expensive, determining the aquifer parameters from VES results is a low cost-effective alternative. In a studied area four profiles were conducted, each profile includes of five VES points of measurements and each VES was interpreted using both manual and IPI2 win program for determining aquifer depth and resistivity. The deduced VES results in piramagroon basin are well coinciding with the relation conditions of the continuous adjacent Sharazoor basin. Then the calculated resistivity values range between (37 Ω .m) to (102 Ω .m) are substituted in the empirical porosity-resistivity equation in the study area shows the aquifer porosity distribution along studied area rang between (21 %) to (39 %).

Whereas substituting in the hydraulic conductivity-resistivity equation in the study area will show hydraulic conductivity values range from (1 m/day) to (4 m/day).

The calculated aquifer porosity and hydraulic conductivity values clearly show the applicability of the used equations in the studied area including presence of unconsolidated alluvial deposits of gravel and sand with decreasing of clay content toward both limbs of the uplifted subsurface layers overlying upper part of Middle Tanjero Formation.

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تقيم مسامية وتوصيل المياه للخزان الجوفي من المعادلات التجريبية باستخدام قياسات الجيوكهربائية العمودية في منطقة بيرمكرون – شمال شرق العراق

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الملخص:

يتناول البحث تطبيق طريقة المسح الكهربائي العمودي بترتيب شلمبرجر كأسلوب منخفض التكلفة وأداة حقيقية في استكشاف المياه الجوفية, وهوالاسلوب الأكثر استخداما للمسح الهيدروجيولوجي للحوض الرسوبي. وتستخدم هذه الطريقة بانتظام لحل مجموعة واسعة من مشاكل المياه الجوفية والمعاملات الهيدروليكية مثل مسامية الخزان الجوفي في هذه الدراسة. ولذلك فإن الهدف من هذه الدراسة هو تقييم قابلية تطبيق وموثوقية المعادلة التجريبية التي تربط بين مسامية الخزان ومقاومتها الكهربائية، لهذا الغرض تم اختيارأربع مسارات المسح في منطقة بيرمكرون والمتكونة من تواجد تكوين تانجرو المغطاة بالترسبات الحديثة, ويتضمن كل مسارمن ست نقاط قياس المقاومة الكهربائية. وتم تفسير المتكونة من تواجد تكوين تانجرو المغطاة بالترسبات الحديثة, ويتضمن كل مسارمن ست نقاط قياس المقاومة الكهربائية. وتم تفسير المتكونة من تواجد تكوين تانجرو المغطاة بالترسبات الحديثة, ويتضمن كل مسارمن ست نقاط قياس المقاومة الكهربائية. وتم تفسير المتحنيات المقاومة الكهربائية باستخدام كل من التفسير اليدوي وبرنامج min وتوصيل المياه بالغران الجوفي وكذلك سمكه وعمقه. ومن ثم تم تعويض قيم المقاومة الحسوبة في معادلتي علاقة السامية وتوصيل المياه بالماه بالغزان الجوفي وكذلك سمكه وعمقه. ومن ثم تم تعويض قيم المقاومة الحسوبة في معادلتي علاقة السامية وتوصيل المياه بالماومة. حيث استنتيع من القيم المسامية المستخرجة من تطبيق المادلة المامية من مقاومة الخزان الجوفي (Ω.m (Ω.m)) الى (20.m) و مسامية الخزان تتراوح بين (٪ 21) إلى (٪ 39) وان قيم توصيل المياه للخزان تتراوح بين(10 مال الى (20.m)) الى (20.m) إلى ورجع سبب هذا الازياد من قمة تركيب الطية الواقعة تحت ناحية بير مكرون باتجاه الشمال الى قرية قازياوا وكذلك باتجاه الجنوب الى قرية جفش الى التناقص من قمة تركيب الطين داخل الترسبات المتجوية الميات الخشاة الحديثة الغير متواليا والحمي الوامحمى الواقعة فوق الجزء الملوي نسبة الطين داخل الترسبات المتجوية المتبادات الخشنة الحديثة الغير متماسكة من الرمل والحصى الواقعة فوق الجزء العلوي ينبية الطين داخل الترسبات المتجوية المترسات الخشنة الحديثة الغير متماسكة من الرمل والحصى الواقعة فوق الجزء الملوي

الكلمات المفتاحية: المسامية الخران (¢)، توصيل المياه للخزان (K)، المعادلات التجريبية ل (¢) و (K)، مقاومة الكهربائية العمودية (VES).

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